



PROJECT ROUTE: IL 47 at Burlington Road LIMITS: 750ft NW to 750ft SE of IL 47(Burlington), & 1000ft S to 1000ft N of Burlington (IL47) MUNICIPALITY/COUNTY: Kane County JOB NUMBER: CMM-8003(829) Local Area Section No. 07-00357-00-CH

> PREPARED FOR: District One Bureau of Programming Hydraulics Section

> > DATE: July 2011

PREPARED BY: Burns & McDonnell 1431 Opus Place Suite 400 Downers Grove, IL 60515

#### TABLE OF CONTENTS

#### 1-00 EXISTING DRAINAGE SYSTEM

- 1-01 IDENTIFIED DRAINAGE PROBLEMS
- 1-02 IDENTIFIED BASE FLOODPLAINS
- 1-03 MAJOR DRAINAGE FEATURES

#### 2-00 PROPOSED DRAINAGE SYSTEM

- 2-01 DESIGN CRITERIA
- 2-02 OUTLET EVALUATION
- 2-03 STORM WATER DETENTION ANALYSIS
- 2-04 RIGHT OF WAY ANALYSIS
- 2-05 DRAINAGE ALTERNATIVES
- 2-06 LOCAL AND OTHER AGENCY COORDINATION
- 2-07 PROPOSED DRAINAGE PLAN

#### 3-00 FLOODPLAIN ENCROACHMENT EVALUATION

#### 4-00 ILLINOIS DEPARTMENT OF NATURAL RESOURCES OFFICE OF WATER RESOURCES (IDNR-OWR) PERMIT

- 5-00 Appendix A: Source Data Reviewed
  - Appendix B: Exhibits
  - Appendix C: Correspondence
  - **Appendix D:** Supporting Documents
  - Appendix E: Erosion and Sediment Control
  - Appendix F: Water Quality/BMP White Paper

### LOCATION DRAINAGE STUDY CHECKLIST

Project Route:	IL 47 at Burlington Rod								
Limits:	nits: 750ft Northwest to 750ft Southeast of IL 47 on Burlington; 1000ft South to 1000ft North of Burlington on IL 47								
Municipality/County: Kane County									
Job Number:									
EXISTING DRAINAGE SYSTEM (see Exhibit 1-00a, General Location Drainage Map; Exhibit 1-00b, Existing Drainage Plan)									
X Yes									
1-01.1 E	Description:	Burlington in February of 2009 on all pavement at the ted that the ditches and culverts							
R	Responsibility		Others	(explain)					
A N C	Action (describe re No action is propo culverts will addres	ecommendation sed at this time ss this issue.	i below) .   The regradi	ng of ditches and replacement of					
Γ	Incorporate into the Study as data base (See Section 2-05 and/or Section 2-07)								
[	<ul> <li>Refer to others</li> <li>Bureau of Maintenance</li> <li>Local Agency</li> </ul>								

1-02 IDENTIFIED BASE FLOODPLAINS (see Exhibit 1-02a Flood Boundary and Floodway Map or Flood Insurance Rate Map and Section 3-00) The Flood Boundary and Floodway Map for the County of Kane

was examined for identified base floodplains which were either traversed by or adjacent to IL 47 at Burlington Road.

Floodplains	🗌 Yes	🖂 No
-------------	-------	------

Floodways 🗌 Yes 🖾 No

1-00

#### 1-03 MAJOR DRAINAGE FEATURES (see Exhibit 1-00a)

Major Culvert Crossings 1-03.2

#### Location: Station 98+90

Structure No.:

Hydraulic Report Prepared by N/A

Waterway Information Table Available:  $\boxtimes$  Yes (Exhibit 1-03.1a) No

Narrative Summary:

While this 3 foot wide by 2 foot high box culvert is not considered a major drainage feature in existing conditions, the proposed culvert (2-2.5 foot wide by 2 foot high) has a cross sectional area greater than 7 feet, and is thus considered a major drainage feature. The reason for the increase in size is to reduce the 50 year headwater elevation and reduce the amount of right of way needed for the detention facilities on the upstream ditches. Information on overtopping, freeboard clearance, etc. is included in the Small Culvert Waterway Information Table in Appendix B, as well as in the calculations in Appendix D.

#### Location: Station 105+90

Structure No.:

Hydraulic Report Prepared by N/A

Waterway Information Table Available:

$\mathbf{X}$	Yes	(Exhibit	1-03.1a)
	No		

Narrative Summary:

This existing culvert is a 3 foot wide by 4 foot high box culvert, and as such is considered a major drainage feature. Information on overtopping, freeboard clearance, etc. is included in the Small Culvert Waterway Information Table in Appendix B, as well as in the calculations in Appendix D.

2-00 **PROPOSED DRAINAGE SYSTEM** (X Exhibit 2-00a, Proposed Drainage Plan)

2-01 DESIGN CRITERIA

(X Exhibit 2-01a - Typical Existing Cross Section. Exhibit 2-01b - Typical Proposed Cross Sections

Construction

Reconstruction

Rehabilitation

The intersection is to be reconstructed as a roundabout. The termini of the project are 750 feet northwest to 750 feet southeast of Illinois Route 47 on Burlington Road, and 1000 feet north to 1000 feet south of Burlington Road on Illinois Route 47.

1. Proposed storm sewer conveyance systems will be designed for a 10 year storm frequency with a velocity between 900mm/sec (3 ft/sec) and 3000mm/sec (10 ft/sec). For storm sewers oversized for detention minimum velocity is 2 ft/sec.

Yes No N/A

Justification for non-compliance:

2. Proposed ditches will be designed for a 50 year storm frequency and desirable ditch grades will be no less than 0.5%.



Justification for non-compliance: Ditches will be designed for a 50 year storm frequency. Ditch grades will be a minimum of 0.3% to accommodate detention within the ditches.

3. The roadway edge of pavement at the low grade point in a floodplain area for highways with a DHV of 100 or more shall be a minimum of three feet above design headwater elevation.

☐ Yes ☐ No ⊠ N/A

Justification for non-compliance:

4. It is required that a minimum clearance of two (2) feet be established between the design high water and the low beam elevation of bridge structures. The bottom of the bridge super structure shall not be below the all-time high water elevation for the new freeway and expressway construction.

🗌 Yes	🗌 No	🛛 N/A
-------	------	-------

Justification for non-compliance:

5. The waterway openings of bridges and culverts will be designed for a 50 year storm frequency.

Yes No N/A

Justification for non-compliance:

6. The vertical alignment for curbed pavements will have a minimum grade of 0.3% and a drainage maximum "K" value of 51 (167 English Unit).



Justification for non-compliance: Flat terrain makes minimum profile expensive. Substandard profile section is superelevated as much as 6%. All curbed pavements have a slope exceeding 0.2%. Some uncurbed sections have a flatter longitudinal slope but feature significant superelevation.

A drainage design exception is not required at this stage based on the proposed scope of

work. If the scope of work is changed during the P.S. & E. stage, the appropriate drainage design exemption approval, if any, will be processed through the Hydraulics Section by the District's Bureau of Design.

#### 2-02 OUTLET EVALUATION

Unless otherwise noted below, the various outlets within the limits of the subject improvement were determined to be suitable for continued use under proposed conditions without modifications or the provision of storm water detention.

Unsuitable ou	itlets: Xes	🗌 No	
Location:	Outlet #1, at Station 105+90 Outlet #2, at the East Quadra Road. Outlet #3, at the South Quadra Road.	in IL 47 ant Farm Entrance on the Eas Irant Farm Entrance on the We	t side of Burlington est side of Burlington

#### Source:

Evaluation: **Outlet #1** has a minimal amount of cover over top of it. This outlet will require the provision of storm water detention in order to maintain existing discharges. Further, the existing culvert will need to be extended to accommodate the proposed roadway width in its vicinity.

**Outlet #2** is currently a corrugated metal pipe which is in disrepair. This outlet will also require the provision of storm water detention in order to maintain existing discharges. Further, the farm entrance will be replaced as part of the project, so the pipe will need to be replaced.

**Outlet #3** is currently a corrugated metal pipe which is in disrepair. This outlet will also require the provision of storm water detention in order to maintain existing discharges. Further, the farm entrance will be replaced as part of the project, so the pipe will need to be replaced.

Recommendation: **Outlet #1** – the culvert under IL 47 will be replaced to provide adequate cover over the structure, and also extended to accommodate the roadway widening in its vicinity. In addition, required detention will be provided upstream of the culvert in an oversized ditch restricted in flow by a ditch check.

**Outlet #2** – the culvert under the East Quadrant Farm Entrance will be replaced with a reinforced concrete pipe to provide adequate capacity and strength. Required detention will be provided upstream of the culvert in an oversized ditch restricted in flow by a ditch check.

**Outlet #3** – the culvert under the South Quadrant Farm Entrance will be replaced with a reinforced concrete pipe to provide adequate capacity and strength. Required detention will be provided upstream of the culvert in oversized ditches both upstream and downstream of IL 47. Each of these ditches will be restricted in flow by a ditch check.

Sensitive outlets:

Yes

No 🛛

**Outlet #1** – This outlet is not considered sensitive because it discharges to an open field which does not have any flooding concerns. There are no structures in the vicinity downstream of this outfall.

**Outlet #2** – This outlet is not considered sensitive because it discharges to a roadside ditch which does not have any flooding concerns. There are no structures in the vicinity downstream of this outfall.

**Outlet #3** – This outlet is not considered sensitive because it discharges to a roadside ditch which does not have any flooding concerns. There are no structures in the immediate vicinity downstream of this outfall.

#### 2-03 STORM WATER DETENTION ANALYSIS

This project has been reviewed in accordance with Drainage Manual, Section 1-303.03 "Storm Water Storage".

- 2-03.1 Evaluation
- No storm water detention required
- Storm water detention required

Unsuitable outlets (see Section 2-02)

Location: Detention is required as part of this project for all of the outlets identified in Section 2-02, Outlets 1, 2, and 3.

#### 2-.03.2 Recommendation

Detention P	onds	] Yes	🛛 No				
Storage Pip	es 🗌 Yes	s 🛛	No				
Oversizing	storm sewers/c	litches	🛛 Yes	🗌 No			
For Outlet	#1 at Station	105+90	:				
	470 Cu	. Yds.	50 year st	orm frequency	ý		
	Oversizing sto	orm sewe	ers/ditches	location:			
	Upstream of I Station 624+2 Design Cross D) Proposed in Appendix D release rate is	47 in tl 5 to Sta Section cross se . The re 7.16 cfs	ne North Qu tion 629+00 Key Map ir ections are a equired and S.	uadrant from ) (See Ditch ) Appendix also located actual			
Control structu	re schematics	(see Exl	nibit 2-03.2	a)			
Detailed evalua	ation and supp s 🗌 No	orting ca	alculations	are included in	Appendix D.		
For Outlet Burlingto	: #2 at East Q n Road:	uadrant	Farm Entr	ance on east	side of		
	_40Cu	. Yds.	50 year st	orm frequency	/		
	Oversizing sto	orm sewe	ers/ditches	location:			
	In the East Qu Design Cross Sections are a release rate in	adrant f Section also loca the 50	rom Statior Key Map ir ted in Appe year storm	612+50 to St Appendix D) ndix D. The r is 3.9 cfs.	ation 614+66 ( Proposed Cro equired and ad	(See Ditch )ss ctual	
Control structu	Control structure schematics (see Exhibit 2-03.2a) ⊠ Yes □ No						
Detailed evalua	ation and supp S	orting ca	alculations a	are included in	Appendix D.		
For Outle Burlingto	et #3 at South on Road:	Quadra	nt Farm Ei	ntrance on we	est side of		

174 Cu. Yds. 50 year storm frequency

Oversizing storm sewers/ditches location:

The required storage is provided Upstream of IL 47 in the West Quadrant from Station 630+00 to Station 637+00 (See Ditch Design Cross Section Key Map in Appendix D) and downstream of IL 47 in the South Quadrant from Station 600+00 to 607+00. Proposed cross sections are also located in Appendix D. The required and actual release rate for Subarea 3-1 is 3.1 cfs; Subarea 3-2 is 8.6 cfs; Subarea 3-3 is 2.80 cfs; and Subarea 3-4 is 3.40 cfs,

Control structure schematics (see Exhibit 2-03.2a) ⊠ Yes □ No

Detailed evaluation and supporting calculations are included in Appendix D.

🛛 Yes 🗌 No

#### 2-04 RIGHT OF WAY ANALYSIS

	🛛 Yes 🛛	] No		Additional right of way is required to accommodate the proposed drainage system.
				Location: The limits of the proposed right of way are identified by station and offset in the Intersection Design Study. The first page of the IDS, containing all of the proposed right-of-way information, is included as Exhibit 2-04a. The right of way requirements are based entirely on the need to provide detention.
	☐ Yes [	⊠ No		A drainage easement(s) is required to accommodate the proposed drainage system
2-05	DRAINAGE N	E ALTEF N/A	RNATIVE	ES
2-06	LOCAL AN	ID OTH	ER AGE	NCY COORDINATION (see Appendix C)
		Yes	🗌 No	Local ordinances considered
		Yes	🖂 No	Joint participation
		Yes	🖂 No	Sewer separation
		Yes	🛛 No	Jurisdictional transfer
		Yes	🛛 No	Letter of intent required/processed/approved
		Yes	🗌 No	Coordination completed and comments provided.
	Comments	÷		Kane County Division of Transportation was consulted with respect to compliance with the Kane County Stormwater Ordinance. An email summarizing differences in methodologies was sent to Kane County, and per their direction, all stormwater facilities were designed using IDOT methodologies. The email citing concurrence with this direction is included in Appendix C. In addition, the Village of Campton Hills' Stormwater Ordinance was consulted. They have adopted Kane County's Ordinance. IDOT Coordination and emails from the Bureau of Local Roads are also included in Appendix C.

#### 2-07 PROPOSED DRAINAGE PLAN

2-07.1	Roadway	Drainage	
	🗌 Yes	🛛 No	Utilize Existing Drainage System
			Limits:
			Comments:
	🗌 Yes	🖾 No	Utilize existing storm sewers with minor extensions and/or adjustment of existing drainage structures
			Limits:
			Comments:
	🗌 Yes	🛛 No	Utilize existing combined sewers with minor extensions and/or adjustment of existing drainage structures
			Limits:
			Comments:
	🛛 Yes	🗌 No	Regrade/reestablish existing ditches
			Limits: Throughout the intersection area, as shown in the proposed ditch cross sections and Key Map included in Appendix D.
			Comments: Proposed ditches will also serve as detention areas to compensate for the additional runoff created by the additional impervious areas attributed to the construction of the project.
	🗌 Yes	🛛 No	Regrade/reestablish existing swales
			Limits:
			Comments:
	🗌 Yes	🛛 No	Replace/relocate existing storm sewers
			Limits and sizes:
			Comments:
	🗌 Yes	🛛 No	Replace/relocate existing combined sewers
			Limits and sizes:
			Comments:
	🗌 Yes	🛛 No	Abandon existing storm sewers
			Limits and sizes:
			Comments:

🗌 Yes	🖾 No	Abandon existing combined sewers
		Limits and sizes:
		Comments:
🛛 Yes	🗌 No	Regrade/reestablish/maintain existing outlets
		Limits and sizes: Existing outlet locations will be maintained, with some improvements to the structures themselves, as described in Section 02-02. These outlets are depicted in the Existing Drainage Plan (Exhibit 1-00b), as well as the Proposed Drainage Plan (Exhibit 2-00a)
		Comments:
🛛 Yes	🗌 No	Maintain/replace/extend existing cross road culverts
		Limits and sizes: Existing culvert at Station 98+90 under IL 47 is to be replaced with two 2.5 feet wide by 2 feet high box culverts; existing culvert at Station 105+90 under IL 47 is to be replaced with a 4 feet wide by 2 feet high box culvert. Both of the farm entrance culverts on either side of Burlington at approximately Station 23+00 are to be replaced with a 15 inch culvert.
		Comments:
🛛 Yes	🗌 No	Construct new storm sewers
		Limits and sizes: Within the roundabout area, as depicted in Exhibit 2-07.1. All proposed sewers will be 12". Calculations for this area were based on a Hydraflow model which was prepared for the sewer with the largest tributary area. This model is included in Appendix D. It should be noted that the proposed storm sewer will be constructed as shown in the interim condition, and then construction of the ultimate build out will require cutting back the storm sewers and relocating the interim conditions inlets to the proposed conditions locations, and hooking the interim storm sewer into the relocated inlets in the new Curb and gutter in the ultimate build out condition.
		Comments:
🗌 Yes	🖂 No	Construct new combined sewers
		Limits and sizes:
		Comments:
🗌 Yes	🛛 No	Construct special drainage structures
		Limits and types:
		Comments:
🗌 Yes	🛛 No	Construct new ditches (standard ditches desired)

		Limits:
		Comments:
🗌 Yes	🖂 No	Construct new swales
		Limits:
		Comments:
🗌 Yes	🖂 No	Construct new outlets
		Locations and types:
		Comments:
🗌 Yes	🖂 No	Construct new cross road culverts
		Locations and sizes:
		Comments:
🛛 Yes	🗌 No	Stormwater detention to be provided (see Section 2-03)
🗌 Yes	🛛 No	Compensatory storage for floodway to be provided (See Section 3-00)

2-07.2 Proposed Action for Major Drainage Features (include hydraulic data such as waterway opening, clearance, freeboard, backwater, permitting requirements, etc.)

Replacement of culverts at Station 98+90 and 105+90 as discussed in Section 2-07.2. See also the Small Culvert Waterway Information Tables included as Exhibits 1-03.1a and 1-03.1b, as well as the calculations located in Appendix D.

#### 3-00 FLOODPLAIN ENCROACHMENT EVALUATION

The proposed project has been reviewed in accordance with Executive Order 11988 "Floodplain Management"; Section 26-7.05(d) "Assessment and Documentation of Floodplain Encroachments" as contained in the Illinois Department of Transportation, Bureau of Design and Environment Manual; Drainage Manual; and 17 Illinois Administration Code 3708 "Floodway Construction in Northeastern Illinois."

No Potential Floodplain Encroachment

### 4-00 ILLINOIS DEPARTMENT OF NATURAL RESOURCES OFFICE OF WATER RESOURCES (IDNR-OWR) PERMIT

 $\Box$  Required  $\boxtimes$  Not Required

#### 5-00 Appendix A: <u>Source Data Reviewed</u>

USGS Maps\* - Quadrangle Map and/or Hydrologic Atlas - See Exhibit 1-00

Survey notes\*

Scoping Report\*\*

Proposed Geometrics\*\* (location, originator, date)

- \* On file in the Hydraulics Section
- \*\* On file in the Project and Environmental Studies Section
- \*\*\* Transmitted to the Bureau of Design

#### 5-00 Appendix B: Exhibits

General Location Drainage Map, Exhibit 1-00a Existing Drainage Plan, Exhibit 1-00b Flood Insurance Rate Map, Exhibit 1-02a Station 98+90 Small Culvert Waterway Information Table, Exhibit 1-03.1a Station 105+90 Small Culvert Waterway Information Table, Exhibit 1-03.1b Proposed Interim Conditions Drainage Plan, Exhibit 2-00a Proposed Build Out Conditions Drainage Plan, Exhibit 2-00b Typical Existing Cross Sections, Exhibit 2-01a Typical Proposed Cross Sections, Exhibit 2-01b Control Structure Schematic, Exhibit 2-03.2a Proposed IDS, Exhibit 2-04a Proposed Inlet and Storm Sewer Layout - Interim, Exhibit 2-07.1a

## 5-00 Appendix C: <u>Correspondence</u>

Drainage Evaluation Meeting Minutes

IDOT Flooding records

IDOT Local Roads Coordination

Kane County Direction for methodology

## 5-00 Appendix D: <u>Supporting Documents</u>

#### Calculations

Ditch design with Final Build Out Key Map

Proposed Ditch Cross Sections in each quadrant stationed as per Key Map

## 5-00 Appendix E: Erosion and Sediment Control Data References

	SOURCE LOCATION														
REQUIRED DATA		LDS E	xhibits			-		OS Sectio	ns	-	-	LDS /	Appdx	Hydraulic	
	1-00A	1-00B	1-02A	2-00A	1-01	1-03	2-02	2-03	2-07	3-00	4-00	С	D	Report	Comment
Drainage patterns and areas	х	х		х											
Receiving waters		х		x											
Floodway and floodplain boundries			x												
Identified areas of flooding					х								х		
Critical erosion and siltation areas															N/A
Inlet(s) and Outlet(s)		х		x	х		х	x	х				х		
Off-site flow adjacent to site		х													
Bridge and culvert location & size		x		x			х	x	х				x		
Storm sewer and/or ditch system							x	x	x				х		
Subsurface drainage tile															N/A
Detention facilities							х	х	Х				х		
Existing erosion control facilities															N/A
Compensatory storage area															N/A
Potential erosion/scour problems															N/A
Erosion/scour prevention measures															N/A
High water elevation													х		
Normal water elevation															N/A
Release rates													х		
Runoff coefficients													х		
Peak flows	Ī												х		
Velocities													х		
Commitments															N/A

#### 5-0 Appendix F: Water Quality/Best Management Practices (BMPs) White Paper

Stormwater runoff generated in the project area will be directed to roadside ditches, and discharged at a rate less than or equal to the existing condition. Roadside ditches will be planted with deep rooted native vegetation to encourage infiltration as the runoff is conveyed through the site. Detention will be provided to attenuate the flows leaving the site and to encourage deposition of sediments within the roadside ditches before the runoff leaves the site.

During construction, a sedimentation and erosion control plan will be put in place and maintained on a regular basis to prevent erosion and discourage sedimentation off site. This plan will be developed as part of Phase 2 of this project.





# EXHIBIT 1-03.1A

# SMALL CULVERT WATERWAY INFORMATION TABLE

Route: IL Route 47

Computed By: DEB

Checked By:

Date: 5/2011

Rev. Date:

Section: 07-00357-00-CH

County: Kane

Station: 98+90

Existing Structure Number:

Drainage Area =7.31 acres							
Existing Low Grade Elevation = 1002.73 @ Station 98+90							
Proposed Low Grade Elevation = 1002.94 @ Station 98+00							
Flood	Frequency	Frequency Discharge Headwater E					
	Year	cfs	Existing	Proposed			
10 year	10	17.33	1002.43	1002.16			
Design	50	25.28	1002.95	1002.50			
Base Flood	100	29.61	1003.24	1002.67			
Max Calc	500	34.31	1003.59	1002.86			

10 Year Velocity through Existing Culvert: 4.8 ft/sec

10 Year Velocity through Proposed Culvert: 4.8 ft/sec

Scope of Work

Existing Culvert		Proposed Culvert
Type: 3 ft wide by 2 ft high rc box culvert		Type: 2-2.5 ft wide by 2 ft high rc box culvert
Length: 54 fee	t	Length: 130 feet
U/S Flowline:	1000.73	U/S Flowline: 1000.94
D/S Flowline:	1000.21	D/S Flowline: 1000.18

# EXHIBIT 1-03.1B

# SMALL CULVERT WATERWAY INFORMATION TABLE

Route: IL Route 47

Computed By: DEB

Checked By:

Date: 5/2011

Rev. Date:

Section: 07-00357-00-CH

County: Kane

Station: 105+90

Existing Structure Number:

Drainage Area =13.23 acres				
Existing Low Grade Elevation = 1002.61 @ Station 105+90				
Proposed Low Grade Elevation = 1000.9 @ Station 105+90				
Flood	Frequency	Discharge	Headwater E	levation
	Year	cfs	Existing	Proposed
10 year	10	20.32	1000.51	1000.46
Design	50	27.94	1000.95	1000.83
Base Flood	100	33.02	1001.22	1001.08
Max Calc	500	41.78	1001.66	1001.53

10 Year Velocity through Existing Culvert: 2.68 ft/sec

10 Year Velocity through Proposed Culvert: 2.91 ft/sec

Scope of Work

Existing Culvert		Proposed Culvert
Type: 3 foot w	de by 4 foot high rc box culvert	Type: 4 feet wide by 2 feet high rc box culvert
Length: 61 fee	t	Length: 90 feet
U/S Flowline:	998.61	U/S Flowline: 998.9
D/S Flowline:	997.54	D/S Flowline: 997.3



	1	and the second of	1	30.00
× 1005.3	× 1004.7		× 1000.7	
		1008.5		
			1000	
× 1006.7	1008.6	EXIST. CURVE 1L3		× 999.2
×1005.6	X	$ \begin{array}{l} \Delta = 78^{\circ} \ 45' \ 13'' \ (LT) \\ 0 = 3^{\circ} \ 09' \ 59'' \\ R = 1.809.57' \\ T = 1.485.17' \\ L = 2.487.27' \\ \end{array} $	and	- Sala
× 1005.0	1006.4	E = 531.43' e = T.R. = S.E. RUN = P.C. STA. = 105+09.15		No. of Concession, Name
		* <b>f\$002;2</b> A. = 129+96.42 × 1	202.2	
		× 1002.4		\ x <b>ı</b>
1008.5	× 100	и.о	× 1005.4	
	× 1001.0			× 1002.2
10010			and a second	
1000	× 99 × 999.4		1000	× 1001.1
998.5				and the second
	× 996.6	× 996.9		× 998.7
¥ 996.8	and the second			1
¥ 996.8	× 996.7	× 995.7	× 996.4	× 997
× 997.1 ¥ 996.1	.309 ×	2	·····	
994.8	× 995.1	× 995.4		
× 994.8 × 994.5	× 994.	.5	× 994.7	995.2
× 994.7	× 994.5	NORTI	H ×994.5	The state
× 996.1 × 994.7	× 201 0	× 994.5	×	995.8
uo 5994.3	99	4.5	, _, _,	996.4
× 994.7	100 × 994.5	'′ 200′ ≍ <b>995.0</b>		100′ ≪ 995.5
5.8 > 995.1 > 994.8	× 99	6.1	× 995.2	
× 994.4	× 994.5	× 996.4	Statistics.	× 994.4
995.0		EGEND		
× 994.1		OUTLET		,
× 994.7			EU SHEET FLOW	
× 995.2 × 998.4		AREA T	O OUTLET	-
	-+	<ul> <li>PROPOS</li> </ul>	ED SWALE	
× 993.3	~	<ul> <li>PROPOS</li> </ul>	ED DITCH	
8.0 992.6		PROPOS	ED CULVERT	
×998.1		- EXISTIN	IG R.O.W.	
× 991.0		- PROPOS	ED R.O.W. /	
>> 2010		GRADIN	G LIMIT	
2-00a AD AT IL 47	F.A. S	ECTION	COUNTY TOT SHEE	AL SHEET TS NO.
IAGE PLAN (INTERIM DESIGN)			CONTRACT NO.	



× 1005.3			111. a	
		× 1004.7		<b>G</b> 2
.2 × 1006.8 × 1005.0	) ×1004.6		× 100	0.7
			1008.5	
- 1008.7		X I		)T
or × 10	105.7	MAN		
×1008.7	10006.6			× 999.2
21×10	105.6		× 1002.2	9331
<u>-</u>		1005.4		
× 1006.0		× 1002.2	¥ 1002 9	
			~ 100mm	
		XXX	1002.4	
× 1008.5	Joda a		, × 1005.4	
		× 1001.0		
	× 1001.0			× 1002.2
30000	1001.0	and the second second		
1000	and the second	∑ 996.7	and the second	
NY NY	× 999.4	and the second s	1000	1001.1
· × 998.5	and the second se			The subscription of the su
		× 996.9		
	× 996.6	8.6	× 996.9	× 998.7
× 996.8				and the second s
× 998.8	× 996.7	× 995.7	× 996.4	
<b>▲</b> 997.1	0051	× 995.2		× 997.1
Ŷ.	830.1		Annual Contraction of the Contra	
× 994.8	× 995.1		\$900.4 M	
DRTH≍ ‱ ×	994.5	× 994.5	× 994.7	
× 994.7	× 994.5		× 994.4	× 995.2
		×994.8 N	ORTH × 994.5	
× 995.1	. 394.7		~	× 995.8
5.8 🔀 994.3	× 994.3	Section of	A 994.5	996.4
× 995.1	994.7	994.5	2004	
× 994.7	× 994.5	100	× 995.0	400 X 995.5
5.8		× 995.1	× 995.2	
995.1	in <b>1714.8</b>			× 994.4
× 994.4	994.5	× 996.4	and the second second	
× 995.0	S. BAS	LEGEND		
> ×994.1		<b></b> OL	ITLET	
$\cap$			OPOSED SHEET	FLOW
× 994.7	11-2-2-2	ĪN	TERPRETED DRA	
X 993.4		AF	EA TO OUTLET	
	aular 1	<del>_+→</del> PF	OPOSED SWALE	
	1 million			
X 993.3		v-> Ph	UPUSED DITCH	
8.0		PF	OPOSED CULVE	RT
× 992.6				
×998.1		——— EX	ISTING R.O.W.	
× 991.0		— — PF	OPOSED R.O.W.	/
× 991.3		GF	ADING LIMIT	
-00b	F.A.	SECTION	COUNTY	TOTAL SHEET
	RTE.	SECTION	COUNTY	SHEETS NO.
NU AI IL 4/				









\$DATE\$ \$FILEL\$ \$SCALE\$ DATE NAME SCALE NAME PLOT FILE PLOT USER

BDE-9409



![](_page_31_Figure_0.jpeg)

\$DATE\$ \$FILEL\$ \$SCALE\$ \$USER\$

DATE = NAME = SCALE = NAME =

PLOT FILE PLOT USER

INTERSECTION DESIGN STUDY           FAS         ROUTE         104         (BURLINGTON RD)           FAP         ROUTE         326         WITH         (IL ROUTE 47)           SEC. NO. 07-00357-00-CH         H 1"=50'         COUNTY KANE           SCALE         V 1"=10'         COUNTY KANE           SJN :         PROJ. NO.	
SEC. NO. 07-00357-00-CH H 1''=50' SCALE <u>V 1''=1</u> 0' COUNTY <u>KANE</u> 5JN : PROJ. NO	INTERSECTION DESIGN STUDY          FAS       ROUTE       104       (BURLINGTON RD)         FAP       ROUTE       326       WITH
	SEC. NO. 07-00357-00-CH           H 1''=50'           SCALE         V 1''=10'           COUNTY KANE           SJN :            PROJ. NO.

![](_page_32_Figure_0.jpeg)

![](_page_33_Figure_0.jpeg)

Approach	<u>NB IL 47</u>	<u>NWB</u> Burlington	<u>SB IL 47</u>	<u>SEB</u> Burlington
	147'	124'	147'	124'
	74'	89'	74'	89'
	220'	N/A	220'	N/A
	75'	79'	75'	79'
	115'	66'	115'	66'

![](_page_34_Picture_0.jpeg)

![](_page_35_Figure_0.jpeg)

![](_page_36_Figure_0.jpeg)

I.D.S. SHEET B1 OF 8

BDE-9409

![](_page_37_Figure_0.jpeg)

![](_page_38_Figure_0.jpeg)

CURVE BURLEB-1 PI STA. = 116+37.81 $\Delta$ = 3° 39′ 01″ (RT) D = 2° 51′ 53″ R = 2,000.00′ T = 63.73′ L = 127.42′ E = 1.02′ P.C. STA. = 115+74.08 P.R.C. STA. = 117+01.50	CURVE BURLSEB-2 PI STA. = 117+70.76 $\Delta$ = 16° 35′ 31″ (LT) D = 12° 03′ 44″ R = 475.00′ T = 69.26′ L = 137.55′ E = 5.02′ P.R.C. STA. = 117+01.50 P.R.C. STA. = 118+39.05	CURVE BURLSEB-3 PI STA. = 119+12.75 Δ = 65° 18' 13" (RT) D = 49° 49' 21" R = 115.00' T = 73.69' L = 131.07' E = 21.59' P.R.C. STA. = 118+39.05 P.R.C. STA. = 119+70.13
	CURVE BURLSEB-4 PI STA. = 120+10.13 △ = 70° 07′ 09′′ (LT) D = 100° 31′ 08′′ R = 57.00′ T = 40.00′ L = 69.76′ E = 12.63′ P.R.C. STA. = 119+70.13 P.R.C. STA. = 120+39.88	CURVE BURLSEB-5 PI STA. = 121+62.98 △ = 12° 19′ 56″ (RT) D = 5° 01′ 42″ R = 1,139,43′ T = 123.10′ L = 245.25′ E = 6.63′ P.R.C. STA. = 120+39.88 P.T. STA. = 122+85.13
05+40.09 = 09.15, 4.00' LT	20	
	EXIST. C PI STA. D = 3° C R = 1,80 T = 1,48 L = 2,48 E = 551. e = 5.65 T.R. = 4 S.E. RUN P.C. STA P.T. STA	CURVE 47-2 = 119+94.32 45' 13'' (LT) 99' 59'' 99:57' 5.17' 5.17' 7.27' 43' 4 0' = 149' . = 109+09.15 . = 129+96.42
RVE         BURLNWB-1         CUR'           STA.         = 218+65.93         PI         S           = 12°         19'         56'' (LT) $\Delta$ =           = 5°         01'         42''         D         =           = 11.139.43'         R         =         =         123.10'         T         =           = 245.25'         L         =         6.63'         E         =         5.5.4.         =           . STA.         = 217+42.83         P.R.I         C. STA.         =         219+88.08         P.R.I	VE BURLNWB-2 STA. = 220+28.08 70° 07' 09'' (RT) 100° 31' 08'' 57.00' 40.00' 69.76' 12.63' C. STA. = 219+88.08 C. STA. = 220+57.84	$\frac{CURVE BURLNWB-3}{PI STA. = 221+31.53}$ $\Delta = 65^{\circ} 18' 13'' (LT)$ $D = 49^{\circ} 49' 21''$ R = 115.00' T = 73.69' L = 131.07' E = 21.59' P.R.C. STA. = 220+57.84 P.R.C. STA. = 221+88.91
CURVE BURLNWB-4 PI STA. = 222+58. △ = 16° 35′ 31″ (F D = 12° 03′ 44″ R = 475.00′ T = 69.26′ L = 137.55′ E = 5.02′ P.R.C. STA. = 221+ P.R.C. STA. = 223-	$\begin{array}{rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$	B-5 +90.19 (LT) 223+26.46 4+53.88
	INTERSE <u>FAS</u> ROUTE <u>FAP</u> ROUTE SEC. NO. <u>07-00</u> SCALE <u>1"=75'</u> SJN :	CCTION DESIGN STUDY           104         (BURLINGTON RD)           326         WITH           (IL ROUTE 47)           357-00-CH           COUNTY KANE           PROJ. NO. CMM-8003 (829)           I.D.S. SHEET B3 OF 8

![](_page_39_Figure_0.jpeg)

\$DATE\$ \$FILEL\$ \$SCALE\$ \$IISFR\$

0 0 0

DATE NAME SCALE NAME

PLOT FILE PLOT USER

![](_page_40_Figure_0.jpeg)

![](_page_41_Figure_0.jpeg)

<u>NWB</u> Burlington	<u>SB IL 47</u>	<u>SEB</u> Burlington
108'	254'	108'
103'	128'	103'
N/A	504'	N/A
85'	85'	85'
73'	172'	73'

![](_page_42_Figure_0.jpeg)

![](_page_43_Figure_0.jpeg)

![](_page_44_Figure_0.jpeg)

![](_page_45_Figure_0.jpeg)